Predictors of Urban Variable Source Area: A Cross-Section Analysis of Urbanized Catchments in the United States

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Predictors of Urban Variable Source Area

TITLE

Predictors of Urban Variable Source Area: A Cross-Section Analysis of Urbanized Catchments in the United States

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ABSTRACT

Many studies have empirically confirmed the relationship between urbanization and changes to the hydrologic cycle and degraded aquatic habitats. While much of the literature focuses on extent and configuration of impervious area as a causal determinant of degradation, in this article I do not attribute causes of decreased watershed storage on impervious area \textit{a priori}. Rather, adapting the concept of variable source area (VSA) and its relationship to incremental storage to the particular conditions of urbanized catchments, I develop a statistically-robust linear regression-based methodology to detect evidence of VSA-dominant response. Using the physical and meteorological characteristics of the catchments as explanatory variables, I then use logistic regression to statistically analyze significant predictors of the VSA classification. I find that the strongest predictor of VSA-type response is the percent of undeveloped area in the catchment. Characteristics of developed areas, including total impervious area, percent developed open space, and the type of drainage infrastructure do not add to the explanatory power of undeveloped land in predicting VSA-type response. Within only developed areas, I find that total impervious area (TIA) and percent developed open space both decrease the odds of a catchment exhibiting evidence of VSA-type response and the effect of developed open space is more similar to that of TIA than undeveloped land in predicting VSA response.
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Different types of stormwater management infrastructure, including combined sewer systems (CSS) and infiltration, retention, and detention infrastructure are not found to have strong statistically significant effects on probability of VSA-type response. VSA-type response is also found to be stronger during the growing season than the dormant season. These findings are consistent across a national cross-section of urbanized watersheds, a higher resolution dataset of Baltimore Metropolitan Area watersheds, and a subsample of watersheds confirmed not to be served by (CSS).

KEYWORDS

Rainfall-runoff ratio; Variable Source Area (VSA); Effective Impervious Area (EIA); urbanized watersheds; regression analysis

1 INTRODUCTION AND BACKGROUND

Research has long shown the link between urbanization and degraded water quality and aquatic habitat (Hammer, 1972; Hatt et al., 2004; Newall and Walsh, 2005). For managers of urbanizing watersheds, one key indicator of negative hydrological change has been impervious surface area. Instead of subsurface flows that are typically the dominant response to rain events in humid catchments, the hydrologic response in urbanized watersheds becomes dominated by surface runoff (Leopold, 1968; Arnold and Gibbons, 1996). Increased surface runoff occurs when impervious surfaces in the form of roofs, parking lots, roads and sidewalks prevent precipitation from infiltrating to the underlying soil. The result is a “flashier” runoff-response,
which leads to flooding and erosion and sedimentation of natural water bodies (Booth and Jackson, 1997; McBride and Booth, 2005).

Impervious surface area has emerged as a key indicator of impaired aquatic habitat for watershed managers and urban planners for its ease of conceptual understanding, but research has shown that impervious surface area alone is not sufficient for understanding underlying mechanisms of hydrological response and degradation (Harbor, 1994; Brabec, 2002; Shuster et al., 2005). One key distinction when trying to quantify impervious surface is the functional difference between Total Impervious Area (TIA) and Effective Impervious Area (EIA). Underlying the concept of EIA is the idea that degree of connectivity of impervious surface area is important in addition to the total magnitude of impervious area (McBride and Booth, 2005; Shuster et al., 2005; Alberti and Booth, 2007; Moglen and Kim, 2007). Emphasis on hydraulic connectivity implies that pervious surfaces could also function similarly to impervious surfaces and hydrologic response is dependent on antecedent moisture of underlying soils, slope and connectivity to impervious surfaces. Alternatively, impervious surfaces that are not hydraulically connected to the drainage network may not be considered EIA. This latter concept is the principle behind run-on infiltration stormwater management techniques in urbanized areas, which aim to “disconnect” impervious areas, reduce peak flows and volumes, and increase baseflows to local streams (Miles and Band, 2015).

Researchers have approached quantifying EIA from TIA in different ways, including using empirical conversion factors, field surveys, and sensitivity analyses, but there is general
agreement that EIA, rather than TIA more closely represents the physical process of hydrological impact on flow regimes (Alley and Veenhuis, 1983; Dinicola, 1990; Booth and Jackson, 1997; Brabec, 2002; Shuster et al., 2005; Knighton et al., 2013; Palla and Gnecco, 2015).

Hydraulic connectivity has not only been shown to be one of the most sensitive parameters in urban hydrological modeling, resulting in modeled peak discharge variations of up to 265% in some cases (Lee and Heaney, 2003). It is also among the parameters estimated with the most uncertainty in urban hydrological modeling (Moglen and Kim, 2007; Knighton et al., 2013).

Others have suggested that overemphasis on connectivity of impervious area (EIA vs TIA) detracts from important changes to soil porosity, vegetation, imported water and other water infrastructure that urbanization has on hydrologic response and catchment water balance (Brandes et al., 2005; Meierdiercks et al., 2010a; Hamel et al., 2013).

In this study, I do not assume impervious area as the dominant causal factor for flashy hydrologic response. Instead, I develop a robust statistical methodology to classify urban catchments into two groups: those dominated by VSA-type response, and those dominated by Hortonian-type response. Based on the classification, I address the following questions:

1.) How does undeveloped land compare to land development variables in explaining the presence of VSA-response?

2.) How does a higher fraction of developed (low density) open area in urban areas influence VSA?
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3.) How does stormwater management infrastructure, such as proximity to a combined sewer outfall, or presence of detention/retention-based stormwater management guidelines affect the probability of VSA-response?

This study contributes to the existing literature by providing empirical evidence of the development-specific characteristics associated with VSA-type response using a cross-section analysis of 119 unique urbanized catchments.

2 URBAN VARIABLE SOURCE AREA

In the Hortonian model of runoff generation, runoff occurs when infiltration rates are exceeded by rainfall intensities. This differs from runoff generation in humid regions, which occurs by subsurface storm flow and saturation excess overland flow (Dunne and Black, 1970; Dunne et al., 1975; Dunne, 1978). Consideration of antecedent soil moisture and differential contraction of saturated areas between storm events led to the “variable source area” (VSA) concept of runoff generation. VSA emerged as an important model describing event-to-event, non-constant runoff contributing areas in undisturbed humid regions (USFS, 1961; TVA, 1965; Hewlett and Hibbert, 1967; Dunne et al., 1975).

Subsequent empirical research has shown that site-specific conditions such as high soil conductivity, steep slopes, mid-slope or downslope positions within the watershed and seasonality affect presence of the VSA condition (McGlynn and McDonnell, 2003; Jencso et al., 2009). In mountainous, alpine forested and agricultural catchments, runoff is first generated in
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riparian zones, and riparian-hillslope connectivity increases under wetter conditions (McGlynn
et al., 2004; Ocampo et al., 2006; James and Roulet, 2007; Wenninger et al., 2008). Monitoring
patterns of soil moisture spatial extent has shown a clear thresholding relationship between
antecedent wetness and rainfall and storm runoff (Detty and McGuire, 2010; Penna et al., 2011).
Event-based rainfall runoff ratios also support threshold relations in subsurface stormflow and
that subsurface flow is a dominant source of runoff (Tromp-van Meerveld and McDonnell,
2006). While the VSA model has been called into question for its ability to apply to all situations
(McDonnell, 2003), it still remains attractive for its ability to conceptualize non-constant ratios in
the rainfall-runoff transformation.

In the study of urbanized catchments, land-use change and other human modifications
to catchments has resulted in both better identification of specific processes and confounded
sources of observed non-constant contributing area and thresholding effects. There has been
significant interest in examining the effects of impervious surface area, infrastructure and
developed open space associated with urbanization on increased hydraulic connectivity at the
catchment scale. Placement and configuration of imperviousness within a catchment can have a
significant influence on downstream response (Mejía and Moglen, 2010). Locations and
configuration of conventional conveyance (Tague and Pohl-Costello, 2008; Meierdiercks et al.,
2010a; Ogden et al., 2011), infiltration-based (Gobel et al., 2004; Easton et al., 2007; Miles and
Band, 2015) and detention-based (Smith et al., 2015) stormwater management infrastructures
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also influence incremental connectivity in hydrologic response of a catchment under varying event depths.

Contrary to commonly held beliefs about limiting imperviousness of development in order to avoid negative changes in hydrologic regime, studies indicate that developed open space can also have limited ability to prevent flashy response. Reasons for this include the limited infiltrative capacity of compacted soils (Smith and Smith, 2015), high proportion of runoff response attributed to shallow subsurface flow under residential lawns (Wigmosta and Burges, 1997), subsurface saturation due to leaky water distribution infrastructure (Lerner, 2002), and decreased evapotranspiration associated with vegetation change (Bhaskar et al., 2015).

Figure 1 shows an adaptation of the VSA to include urban run-on from impervious areas and other potential sources of impacts to soil saturation in urbanized catchments (Miles and Band, 2015). As shown in Figure 1, urban VSA response is associated with incremental connectivity of conveyance infrastructure, impervious areas, and soils and pervious areas.

[Figure 1 (a) Dunne et al.’s original conceptualization of runoff generation process and sources of variable source area. (b) Conceptualization of potential stormwater fates in low to medium density urbanized watersheds. (c) Conceptualization of runoff generation process in low to high density urbanized watershed and potential causes of observed variable source area, including infrastructure storage and leakage. (a) and (b) reproduced from Miles and Band (2015), used with permission from Wiley.]
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Analyses of empirical rainfall-runoff relationships from urbanized catchments have revealed that for smaller storms (< 38.1 mm or 1.5 inches), runoff depths as a fraction of the rainfall depths correspond closely to the EIA of the catchment. However, this relationship is less reliable for larger storms (Doyle and Miller, 1980). Regression-based analyses of the relationship between rainfall and runoff depths have been used to delineate the sequentially gained hydraulic connectivity of EIA, TIA and pervious areas respectively and to estimate their proportions within the catchment area (Boyd et al., 1993, 1994; Goldshleger et al., 2012; Loperfido et al., 2014; Ebrahimian et al., 2016). Studies which examine changing ratios between rainfall depth and runoff depth within a catchment all share a common interpretation that the variable proportion of area contributing to the hydrologic response is dependent on the total depth of rainfall.

This study aims to determine the significant predictors of VSA hydrologic response across urbanized catchments using regression analysis. Previous studies suggest that both impervious surface and land development in general (including seemingly pervious areas) will result in the dominance of Hortonian flow over VSA, while lower levels of development will result in the dominance of VSA over Hortonian overland flow (Miles, 2014). In urbanized catchments with high levels of impervious surface, we expect the contributing area from these catchments to correspond to the fraction of the catchment area that is composed of impervious area.
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In VSA-dominated catchments, we expect a nonlinear relationship between rainfall and runoff. As rainfall depths increase or rainy periods are prolonged, we expect some areas within the catchment area to incrementally lose capacity to store and infiltrate precipitation as storage thresholds are exceeded. This will lead to an increasing slope in the relationship between rainfall and runoff as cumulative rainfall depths increase. It should be noted that the conceptualization presented in this work (Figure 1), departs from the Dunne VSA model in that it includes both runoff production processes (saturation excess and infiltration excess) and other factors specifically of interest in urbanized catchments that influence observable nonlinearity at discrete downstream streamflow measurement locations, such as the presence of CSS or other stormwater management infrastructure.

3 METHODS

Broadly, my methodology involves three steps. First, I perform hydrograph separation to create a dataset of paired event rainfall-runoff depths for each catchment in the analysis. Second, I develop a statistical methodology to detect the presence of nonlinearity in the rainfall-runoff relationship for each of the catchments, using the rainfall event data. Lastly, I use logistic regression to estimate the effects of the catchments’ characteristics on VSA-type response.
3.1 Data

3.1.1 National-level datasets

Catchments for the analysis were selected from stream gauge flow monitored by the USGS, with characteristics included in the GAGES II database. GAGES II was developed by the USGS to provide users with an exhaustive set of geospatially-specified catchment characteristics corresponding to a large number of gauged watersheds. The database includes both “reference” watersheds, which are minimally influenced by human activity, and watersheds that represent a range of hydrologic conditions including urban development intensity (Falcone et al., 2010).

For a catchment in the national-level dataset to be included in this study, I used three criteria. First, the catchment had to be at least 50% developed according to the National Land Cover Dataset urban development classification. Second, the stream gauge had to be located within a 15-mile radius of an airport-based precipitation gauge having hourly data. Third, the catchment had to have at least 35 rainfall events that resulted in paired rainfall-runoff data. Stream gauge data for GAGES II catchments were downloaded from the USGS website (http://nwis.waterdata.usgs.gov/nwis/sw) using basin identification numbers and date ranges for available flow and precipitation data (Lins, 2012). Precipitation data was obtained from the National Climatic Data Center (http://www.ncdc.noaa.gov/). From these criteria, the study included 91 analysis catchment areas in the contiguous US (shown in Figure 2).

The catchments ranged from 50.49% developed to 99.98% developed. The median level of development was 84.37%. The 30-year (1970-2000) average annual precipitation among the
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study basins ranged from 63.31 cm to 136.80 cm. The drainage areas ranged from 3.70 km² to 505.80 km² with a median drainage area of 85.06 km². The generalized rainfall intensities in centimeters per hour for a 2-year, 1-hour storm event ranged from 4.06 cm (1.6 inches) per hour to 5.59 cm (2.2 inches) per hour.

I added three variables to those in the GAGES II database: (1) distance of the stream gauge location to the nearest (upstream or downstream) active combined sewer outfall; (2) whether the watershed included a community served by a CSS; and (3) a binary variable for whether the city or county in which the stream gauge was located encouraged infiltration, retention, or detention-based stormwater management practices at the time of the study. Geospatial locations of permitted outfalls were extracted from the EPA’s Facility Registry Service (http://www.epa.gov/enviro/facility-registry-service-frs) for all permitted combined sewer outfalls listed by EPA (US EPA, 2004). Promotion of stormwater management practices was determined through an internet search of the name of the city and county in which the gauge was located, followed by the terms “Stormwater Detention, Retention, Green Infrastructure, Infiltration.” Locations for which informational materials were readily available were presumed to be “actively” promoting this type of decentralized infrastructure.

Because CSSs have the potential to confound the results of the VSA classification analysis, I subset the national-level dataset with gauges known to not include any combined sewer systems. Of the 91 national-level catchments, 56 were confirmed not to have CSS within
their boundaries. This subset is hereafter referred to as the “non-CSS dataset” (shown in Figure 2).

[Figure 2. Locations of national dataset and non-CSS dataset analysis catchments]

3.1.2 Baltimore Metropolitan Area (BMA) datasets

The question of spatial heterogeneity in rainfall records was one of the major concerns with the analysis of the national dataset. Others have shown that especially in urbanized areas, where human activity and changes to the natural landscape influence micro-climates and local weather patterns, precipitation measured at one discrete location can vary significantly from the amount of rainfall at another nearby rain gauge (Shepherd, 2005; Smith et al., 2012). For this reason, the analyses were also performed on a dataset of Baltimore Metropolitan Area (BMA) basins for which there was HydroNEXRAD radar precipitation data available covering the entirety of the gauge’s catchment area. Radar rainfall data processed by the HydroNEXRAD system was obtained at a 1 square kilometer resolution at 15-minute intervals. A multiplicative bias correction value was then used to bias correct basin average time series data for each basin for each 15-minute time period to calibrate HydroNEXRAD data to precipitation records from a diverse network of rain gauges in the Baltimore region (Smith et al., 2012). This procedure
allowed for the use of spatial precipitation records that fell over the contributing area and should be much more accurate than discrete rain gauge data.

[Figure 3. BMA dataset gauge locations and basin boundaries]

After the time-series-averaged precipitation data was obtained for each watershed, the procedure used for pairing rainfall events with flow gauge readings was identical to that of the national dataset. Due to data limitations, only 34 watersheds over 30% developed were left for the study (Figure 3). The watersheds ranged from 31.86% developed to 96.87% developed. The average annual precipitation depth (from 1970-2000) ranged from 107.9 cm to 123.3 cm. The drainage areas ranged from 1.20 km² to 906.60 km². Some watersheds that were not included in the national-level dataset because of a lack of a proximate rain gauge station were included in the Baltimore Metropolitan Area dataset, for which the more accurate radar precipitation data was available. Combined sewer outfall proximity was calculated as for the national-level dataset. Using the same internet search method as was used for the national dataset, all counties for the BMA catchments were determined to have implemented detention, retention or infiltration-based stormwater management policies, therefore the effect of this development characteristic could not be estimated through regression and it was not included in the BMA.
analysis. Between the national and BMA datasets, there were 119 unique catchments included in the study.

### 3.2 Hydrograph Separation and Event Definition

I used the R package ‘EcoHydRology’ to separate the hydrographs into baseflow and quickflow components (Fuka et al., 2014). By visual inspection of the hydrograph separation for a 12.7 mm (0.5 inches) rainfall event and a 38.1 mm rainfall event (1.5 inches) for a few representative watersheds, I determined that a filter parameter of 0.925 and three passes was appropriate to automate baseflow separation across the variation in my analysis catchments. Since some catchments exhibited very little response to rainfall, I defined the start and end of rainfall events from the continuous precipitation record. Events were defined as any length of time that preceded and followed by 96-hour periods of no rainfall in the precipitation record. The implicit assumption of the 96-hour dry period is that localized groundwater mounding or saturation that could contribute to VSA within a catchment would decrease in influence after that period.

To capture the full quickflow component of the hydrograph in the flow record (especially in larger catchments), I added a buffer of 36 hours after the precipitation-defined end time of the event. Through separated hydrograph inspection, I confirmed that the 36-hour period was long enough to capture the quickflow response even from the larger catchments in the dataset. An example of the separation (described below) is shown in Figure 4. The time step for all hydrograph separation was 15 minutes. All flow data were available at least at this resolution. Flow data collected at a higher resolution time step were averaged to 15 minute intervals.
For each event, precipitation depths were summed and paired with cumulative quickflows over the defined event period and normalized by dividing by the catchment area. Thus, for each analysis catchment, a set of paired rainfall-runoff depths for each event was created. An identical process was carried out for the BMA dataset, except that the source of the precipitation data was basin-averaged, bias-corrected HYDRO-NEXRAD data.

[Figure 4. Example of hydrograph separation using R package ‘EcoHydRology’ for the watershed in our sample with the largest drainage area, Salado Creek in San Antonio, TX (drainage area = 505.8 km²). The flow response to a 38.1 mm (1.5 inch) total rainfall depth is shown. The response returns to baseflow conditions within the 36-hour period.]

3.3 Robust Statistical Detection of VSA

Classification of catchments as having dominant VSA processes was based on the statistical detection of nonlinearity in the rainfall-runoff response of the catchment. Statistical significance of nonlinearity was determined through the estimation of the linear model:

\[
\text{runoff}_{ij} = \alpha_i + \beta_1 \text{rain}_{ij} + \beta_2 \exp \text{rain}_{ij} + u_{ij} \tag{1}
\]
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where $\text{rain}_{ij}$ is the precipitation depth of each event $j$ in the time period for catchment $i$ and $\text{runoff}_{ij}$ is the runoff depth corresponding to precipitation event $j$. If the coefficient estimated for $\exp(\text{rain}_{ij})$ was statistically significant, this indicated evidence of nonlinearity in the rainfall-runoff relationship averaged over many events. In the detection of VSA processes, I expected this nonlinearity to be positive.

One problem with the above specification is that it suffers from heteroscedasticity, or non-constant variance in the residuals of the estimated equation. While heteroscedasticity does not bias the estimates of the coefficients in a linear regression, it does result in inefficient estimates of the standard errors of the coefficient estimates. In order to correct the effects of heteroscedasticity on standard error estimates of the coefficients, I log-transformed both the rainfall and runoff data to improve residual distribution. I used the R package ‘sandwich’ to estimate robust standard errors for the coefficients of the log transformed model [1] (Zeileis, 2004). I assigned catchments as VSA-dominant if $\beta_2$ was significant at the $\alpha=0.05$ level, and non-VSA dominant if $\beta_2$ was not significant at the $\alpha = 0.05$ level. The result of this part of the analysis was an assigned binary hydrological response variable for each of the analysis watersheds: linear, corresponding to no evidence of VSA processes ($Z_i = 0$), or nonlinear, corresponding to evidence of VSA processes ($Z_i = 1$).

Seasonal effects have been shown to influence nonlinearity in event-based rainfall-runoff ratios (Smith et al., 2005; Detty and McGuire, 2010; Meierdiercks et al., 2010b). During the growing season, evapotranspiration reduces soil moisture, allowing catchments to recover
volume more quickly between events. We would therefore expect dormant season subsurface conditions to stay wetter longer and to be more associated with a constant response. In order to gain more clarity on potential sources of variation in nonlinear rainfall-runoff ratios, I specified an additional model to test whether growing season rainfall events have a statistically different rainfall-runoff relationship than dormant season events. The dormant season was defined as the months from October – March and the growing season was defined as the months from April – September (Detty and McGuire, 2010).

The above-specified model [1] allowing for an additional effect of seasonality is shown in model [2]:

\[
\text{runoff}_{ij} = \alpha_{si} + \beta_{1i} \text{rain}_{ij} + \beta_{2i} \exp(\text{rain}_{ij}) + \\
\alpha_{2i} \text{g}_{ij} + \beta_{3i} \text{rain}_{ij} \times \text{g}_{ij} + \beta_{4i} \exp(\text{rain}_{ij}) \times \text{g}_{ij} + u_{ij} \tag{2}
\]

where \( g_{ij} \) is a dummy variable equal to one if event \( j \) occurred during the growing season and zero if event \( j \) occurred during the dormant season. If the regression [2] for catchment \( i \) results in significant coefficients \( \alpha_{2i}, \beta_{3i}, \) or \( \beta_{4i} \), this indicates that the rainfall-runoff ratio is statistically different during the growing season than during the dormant season. The model allowing for estimates effects for seasonality [2] was log-transformed in the same way the restricted model [1] was log-transformed.
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The choice of the exponential form of the term capturing nonlinearity is contrasted to the “breakpoint” or threshold conceptualization of nonlinearity that has been applied in other studies (e.g., Loperfido et al. (2014)). The exponentiated form is preferred for its ability to better reflect incremental exceedance of area-based storage within the catchment and thus, incremental hydrologic connectivity of areas to the downstream streamflow response. The choice to discretize the detection of VSA-type response is limited because it does not capture variation in the magnitude of nonlinearity; however, the focus of this analysis was on explaining the VSA process, rather than on predicting runoff magnitudes from rainfall depths.

3.4 Logistic Regression of VSA on Catchment Characteristics

After obtaining the binary VSA (nonlinear) or non-VSA (linear) response classification for each watershed was obtained, logistic regression was used to test which explanatory variables (catchment characteristics) contributed to the probability of a catchment exhibiting a nonlinear response. The probability of VSA-type response is expressed as an inverse logistic function of catchment characteristics in [3]:

$$\text{Pr}(Z_i = 1) = \logit^{-1}(M_i \gamma_i)$$  \[3\]

where $M_i$ is the vector of $k$ characteristics for catchment $i$ ($m_{i1} \ldots m_{ik}$), and $\gamma_i$ is the vector of coefficients for the characteristics of catchment $i$. Of particular interest was estimating the effect
Predictors of Urban Variable Source Area of development and specifically impervious surface on the probability of a catchment exhibiting a VSA-type (nonlinear) response. Other variables tested as part of the vector \( M \) included: average slope, average annual precipitation, number of data observations, size of the drainage area, percent of various land use types, stream order, basin compactness (a measure of elongation), and region. In the final models, theoretically important variables and variables statistically significant at the \( \alpha = 0.10, 0.05 \) and 0.01 levels were included.

To test my hypotheses, I fit [3] using two sets of models for each of the three datasets (national, BMA, and non-CSS only). The first set of models (Models 1A-1C, shown in Table 2) starts with percent undeveloped land as the sole predictor of VSA-type response (Model 1A), then sequentially adds geologic/morphologic controls as predictor variables in the model (Model 1B), followed by other development characteristic controls (Model 1C). The set of geologic/morphologic and meteorological controls in the models included average slope (%), average annual precipitation (cm/yr), and catchment area (km\(^2\)). Watersheds with lower average slopes are expected to exhibit more variability in the saturated zone and from subsurface throughflow, which result in a VSA response (Dunne et al., 1975). Smaller basins are likely to exhibit flashier hydrological response, which may be associated with reduced VSA effects (Smith and Smith, 2015). The meteorological control included was the average total annual precipitation in the watershed. Higher annual precipitation is likely to be positively associated with humid climates that are likely more dominated by VSA processes than by Hortonian flow, all else being equal (Dunne et al., 1975; Miles and Band, 2015). Other development characteristic
controls included percent TIA, percent developed open space, distance to combined sewer outfall and a binary variable for decentralized stormwater management practices. Percent undeveloped land was calculated from the GAGES II database by subtracting low, medium, and high density development and developed open space percentages from 100%. Developed open space is a National Land Cover Dataset (NLCD) classification defined as the percent of the 30m x 30m grids within the watershed that is estimated to have less than 20% impervious cover. Typically, these areas include large-lot single-family housing units, parks, golf courses and landscaped vegetation in developed areas.

In the second set of models, I removed percent undeveloped land as a predictor variable and only include development-type variables (Models 2A-2C, shown in Table 3). Starting with percent impervious area along with the geologic/morphologic and meteorological controls (Model 2A), I add in other development-type variables, for percent developed open space (Model 2B), and distance to combined sewer outfall, and decentralized stormwater management practices (Model 2C). Estimating the effects of development-type variables separately from the percent undeveloped area variable allows us to test how different development types contribute to explaining the variation in VSA-type response and avoid multicollinearity of explanatory variables.

Goodness-of-fit for the logistic regressions was assessed using two methods: McFadden’s pseudo R-squared and a percent-correctly-predicted pseudo R-squared where the cutoff point was defined as the mean of the dependent variable (Wooldridge, 2010).
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likelihood ratio test was used to evaluate whether the inclusion of additional explanatory variables led to statistical improvement of the model’s fit to the data.

4 RESULTS

4.1 Classification

The robust catchment classification methodology resulted in 69 out of 91 total national-level catchments (76%), 21 of 34 total BMA catchments (62%) and 44 out of 56 total non-CSS watersheds (78%) being classified as having statistical evidence of VSA-processes. Among the national dataset basins, those classified as having nonlinear response had an average drainage area of 90.96 km², while those classified as having a linear response had an average drainage area of 66.5 km². T-test results showed that the difference in means was not statistically significant at the 0.05 level (p = .246, n₁ = 69, n₂ = 22). Table 1 shows the estimated linear and nonlinear coefficients and significance according to the robust standard errors for the BMA watersheds (national dataset results are included as supplemental information). Figure 5 illustrates the linear and nonlinear fits for several example watersheds. From these visual inspections of the fits to the data, I determined that the classifications based on the regression specifications and the robust standard error calculations for both the national (and non-CSS) dataset and the BMA dataset were satisfactory.

[TABLE 1: Estimated linear and nonlinear coefficients and robust standard errors for Baltimore Metropolitan Area watersheds]
[FIGURE 5: Example plots of linear and nonlinear relationships between rainfall and runoff (log transformed). Gray areas represent 90% confidence interval of model fit with both linear and nonlinear terms included, using robust standard estimates. The solid line is the predicted relationship with both linear and nonlinear terms included. The dashed line is the predicted relationship with only the linear term included]

In order to determine whether the addition of the dummy variable for growing season and its interaction with the linear and nonlinear components of the regression significantly improved the fit of the model, I employed a heteroscedastic standard errors-robust F-test comparing the fits of the nested models [1] and [2]. In the majority of the catchments in both the national and BMA datasets, there was no significant improvement in model fit by including the dummy variable for growing season (57/91 catchments in the national dataset and 26/34 catchments in the BMA dataset exhibited no significant differences in fit compared to the restricted model [1], where the values of \( \alpha_{2i} \), \( \beta_{3i} \), and \( \beta_{4i} \) are all constrained to the value 0). Of the catchments that did exhibit improvement by incorporating seasonal differences, many estimated individual effects that were insignificant at the 0.05 level for all three additional seasonal terms (9/34 for the national dataset and 2/8 for the BMA dataset).

Among the seasonal models [2] that did exhibit some improvement over the restricted models [1], the interpretation of significantly estimated regression coefficients of may provide some additional insight into the dynamics of urban VSA runoff behavior. Figure 6 shows the classifications of each basin included in this study, first by whether the fit of the model was
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improved with the inclusion season-specific variables, then by the year-round classification as exhibiting evidence of VSA-behavior, and lastly, by significance and signs of estimated season-specific effects. Figure 6 shows that among those catchments for which the addition of the seasonal variables significantly improved fit, 9/20 of the national dataset and 2/8 of the BMA dataset had insignificant effects for all three variables. For both datasets however, the next frequent classification among those with improved models was for non-VSA basins with significant nonlinear behavior during the growing season. This coefficient was estimated as positive in 4/5 national catchments in this category and 2/2 of BMA catchments in this category. Both these findings are in agreement with the present understanding of VSA runoff generation, which suggests that variable source area dynamics would be more pronounced during the growing season, when evapotranspiration allows basins to recover storage volume more quickly (Detty and McGuire, 2010).

[FIGURE 6 Classification of all analysis basins included in this study based on model improvement with inclusion of seasonal controls, significance of nonlinear term (evidence of VSA behavior), and significance and sign of estimated coefficients associated with effect of rainfall-runoff ratio relationship during the growing season (April – Sep). Of catchments whose models were improved by controlling for seasonality and had significant individual coefficients, the highest frequency that appeared were for VSA catchments with positive coefficients for the
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nonlinear seasonal term. This is in agreement with previous theory and findings that VSA response should be more pronounced during the growing season.]

4.2 Predictors of Urban VSA

Table 2 shows the results of the regressions that include the percent of the watershed that is undeveloped as a predictor of the VSA-classification. For the national dataset, percent undeveloped area alone was a significant predictor of a VSA-type response. A 1% increase in the percent undeveloped land within a watershed was associated with a 3.5% increase in the odds of a watershed exhibiting VSA-type response. For all three datasets, including morphological and meteorological controls in Model 1B led to significant improvement over Model 1A. A likelihood ratio test between Models 1B and 1A yielded p-values of 0.035, 0.00087, and 0.0084 for the national dataset, BMA dataset and CSS dataset, respectively. Based on Model 1B, the effect of a 1% increase in undeveloped land was associated with between 5.5% and 10.8% increase in the odds of the watershed exhibiting evidence of a VSA-type response, controlling for slope, precipitation, and catchment area.

[TABLE 2: Results of logistic regression of percent undeveloped land and other controls on probability of VSA-type response]

Adding in controls for development types sequentially did not further statistically improve the model fits for the national or BMA datasets but some improvement was shown with the non-CSS dataset. One model (not shown in Table 2) estimated with the non-CSS
Predictors of Urban Variable Source Area dataset which included percent developed open space, percent undeveloped land and the morphological and meteorological controls (but excluding percent impervious area), did estimate statistically significant results for both undeveloped land and developed open space and this model was shown to be a statistical improvement over Model 1B (p = 0.03053). The effect of undeveloped land was similar to that estimated in Model 1B (9.32%), but the effect of developed open space was estimated to be -8.232% (p = 0.0463). In contrast, the effect of impervious area is not significant at the $\alpha = 0.05$ level when included with undeveloped area with any of the datasets. This suggests that developed open space functions more similarly to what we would expect from impervious area, and that this effect is most prevalent watersheds that do not have CSS.

Model 1C, which also includes percent impervious area as an explanatory variable, showed slightly significant (p = 0.09) improvement over Model 1B for the non-CSS dataset, but none of the development variable coefficients were estimated to be statistically significant from zero. Model 1C exhibited the problem of rather high variance inflation factors for multiple variables for all three datasets. High VIFs are an indication of multicollinearity between the explanatory variables. Generally, VIF values >10 result in unreliable estimates (Kutner et al., 2004). When percent TIA was removed from Model 1C for the non-CSS dataset, all VIFs fell below 2, suggesting that the source of collinearity was between percent undeveloped and TIA and percent developed open space and TIA, and not between percent developed open space and percent undeveloped.
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A second set of models excluded the percent undeveloped variable to avoid multicollinearity and focus on the effects of percent impervious area, which is commonly identified as the strongest factor in decreased catchment storage and flashier hydrologic response (Table 3). The effect of impervious surface area was not found to be a significant predictor of a VSA-type response until other contextual factors were controlled for. Adding morphologic and meteorological controls (Model 2A), a significant effect was only estimated with the BMA dataset. A 1% increase in the percent impervious area was associated with an 11.1% decrease in the odds of a VSA-type response. TIA only became statistically significant for all three dataset after also controlling for percent developed open space (Model 2B) and a likelihood ratio test also indicates that the model improvement over 2A is statistically significant (p-values for the improvement of Model 2B over Model 2A were 0.01276, 0.08941, and 0.001745 for the national, BMA, and non-CSS datasets, respectively). These models estimated between an 8.0% and 17.9% decrease in the odds of VSA-type response associated with a 1% increase in TIA.

[Table 3: Results of logistic regression of development types and other controls on probability of VSA-type response]
The effect of developed open space was nearly equal in magnitude to that of TIA. A 1% increase in developed open space was associated with between 8.6% and 15.2% decrease in the odds of a VSA-type response. Adding additional variables representing type of development, such as distance to combined sewer outfall and presence of a retention, detention or infiltration-based stormwater management program (Model 2C), neither significantly improved model fit nor resulted in additional significant estimated effects (likelihood ratio test p values for improvement of Model 2C over 2B were 0.553 and 0.308 for the national and BMA datasets, respectively). Model 2C for the national dataset had acceptable VIF values, and controlling for the distance to the nearest combined sewer outfall and presence of distributed stormwater infrastructure resulted in little change to the estimated effects of TIA and developed open space, demonstrating stability of the model. The two models that showed statistically significant improvements—Models 1B and 2B—had similar goodness-of-fit measures and estimated effects of significant controls, further increasing confidence that the results were not spurious.

5 DISCUSSION

5.1 Effect of undeveloped land compared to land development variables in explaining VSA response

The results from models that included undeveloped land as an independent variable show that in general, development type variables add little compared to the explanatory power of undeveloped land for predicting VSA response. This is especially true when morphologic
Predictors of Urban Variable Source Area and meteorological controls are included. When no additional controls are included in the regressions, the effect of undeveloped land is marginally significant, while that of TIA is not significant. Only when controls for watershed morphologic and meteorological conditions do TIA become a stable predictor of VSA-response. This result is important considering the attention that impervious area as a singular metric has been given over the years, especially for land use planning purposes. The conditional significance of impervious area highlights the need to incorporate contextualizing factors into the understanding of catchment-scale hydrological response.

### 5.2 Effect of open space in urban areas on VSA response

As expected, the effect of TIA on VSA-response is negative: a 1% increase in TIA within the watershed is associated with between 8.0% and 17.9% decrease in the odds of detection of a VSA-type response, controlling for other factors. Less expected is that developed open area (low density development) also has a negative effect on VSA-type response almost equal in magnitude to TIA. This suggests that on average, developed pervious area is also associated with Hortonian-flow dominated responses compared to undeveloped areas, a result that has also been confirmed by others (Smith et al., 2015). For land use planners, this means it is not enough to limit imperviousness of new development. In order to preserve VSA-type response, it is necessary to limit even low-density development. TIA is highly correlated with overall development levels (Pearson’s rho = 0.78, 0.80, and 0.91 for the national, BMA and non-CSS datasets, respectively), which explains why this particular metric may have been useful for land
Predictors of Urban Variable Source Area LIM use planners in the past. Developed open space, which was shown in this study to add significantly to the explanatory power of TIA, is not correlated with overall development (Pearson’s rho = -0.13, -0.09, 0.15 for the national, BMA and non-CSS datasets, respectively). This weak correlation, along with the relative invisibility of runoff generation on pervious surfaces compared to impervious surfaces, may explain why the effect of developed open space has been overlooked.

There are several possible explanations for why developed open space has a negative effect on VSA-type response. Developed open space in the NLCD is defined as development that is less than 20% impervious, so these areas could still contain roads and drainage infrastructure that increase hydraulic connectivity. Urban pervious surfaces could have very little storage due to compaction and localized subsurface saturation due to lawn watering and leakage and therefore lead to saturation overflow conditions even during very small events (Lerner, 2002; Bhaskar and Welty, 2012; Smith and Smith, 2015). Although this process is physically more similar to Dunne’s VSA concept of saturation overland flow, if storage is minimal, the hydrological response at this level of analysis is indistinguishable from Hortonian overland flow.

5.3 Effects of stormwater management infrastructure on VSA response

In the national dataset, no coefficients estimated for stormwater management control variables had statistically significant effects, and distance to CSO in the BMA dataset had only a
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marginally significant negative effect on VSA response. Many urban areas in the Northeast and Midwest US are served by combined sewer systems that collect wastewater and stormwater runoff within the same system. During small rain events, these collection systems do not discharge directly to streams, but direct all flows to the wastewater treatment plant, after which, runoff generated in one catchment may be discharged in another. The presence of this kind of infrastructure might suppress the detection of runoff response in highly urbanized areas, mitigating some of the negative effect of high levels of impervious surface in urbanized areas and resulting in decreased (less negative) effects on the probability of VSA compared to suburban areas. The more negative effect of developed open space estimated from the non-CSS dataset offers some supporting evidence that this is true: among watersheds in which runoff is not intercepted by wastewater collection and treatment systems, there is more of a Hortonian-type hydrological response. The data used in this analysis and the formulation of urban VSA include both runoff generation processes and the effects of intermediary structures that could confound the detection of a non-constant rainfall-runoff relationship (Figure 1). However, previously demonstrated empirical evidence that variable source dynamics are more pronounced during summer months were also supported. It should be noted however, that the implications of a “VSA” type response that results from runoff being sent to a wastewater treatment plant during small storms but discharging runoff during large events has very different implications for watershed management than more natural VSA runoff production processes. Estimating the effect of retention, detention and infiltration-based stormwater
Predictors of Urban Variable Source Area management practices from the presence of guidelines including these practices does not necessarily reflect extent of implementation. However, previous research has shown that despite being constructed with modern detention and retention ponds, developed basins in Maryland still functioned more similarly to basins without such infrastructure than to an undeveloped, forested basin (Meierdiercks et al., 2010b).

There are limitations of the data used in this analysis. While the GAGES II dataset is valuable because it allows for a cross-sectional analysis of many watersheds across the US, the resolution of land cover and precipitation data is too low to distinguish among specific physical processes of localized runoff generation. The particular processes and pathways within urbanized catchments ideally should be assessed in the field, and therefore, the conclusions of this study should be understood as the ‘average’ effects of the covariates included in the regressions, as measured at the stream gauge. It could be that issues of resolution among the urbanized catchments studied may mask the specific connectivity conditions of ‘developed open space.’

6 CONCLUSIONS

This study confirms the need to move away from impervious surface as a singular metric for hydrological response, but has particular implications for land use planners and watershed managers. Previous emphasis on limiting imperviousness of new development suggests that low density, suburban development results in less disruption of hydrological
response because of the presence of open space to mitigate flows. This study provides evidence that developed open space functions more similarly to impervious area than it does to natural areas, and shows that there is no evidence that developed open space promotes VSA dynamics. This finding may provide watershed managers and land use planners with additional rationale to promote higher density urban development or redevelopment and preserve naturalized areas rather than develop at low densities with more developed open space. It also implies that bulk lot coverage or zoning regulations that limit imperviousness but do not specifically address preservation of naturalized vegetation or native, undisturbed soils should be reexamined.

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Figure 1 (a) Dunne et al.'s original conceptualization of runoff generation process and sources of variable source area. (b) Conceptualization of potential stormwater fates in low to medium density urbanized watersheds. (c) Conceptualization of runoff generation process in low to high density urbanized watershed and potential causes of observed variable source area, including infrastructure storage and leakage. (a) and (b) reproduced from Miles and Band (2015), used with permission from Wiley.
Figure 2. Locations of national dataset and non-CSS dataset analysis catchments

Legend
- non-CSS dataset pge location
- National dataset pge location

279x215mm (300 x 300 DPI)
Figure 3. BMA dataset gage locations and basin boundaries
279x215mm (300 x 300 DPI)
Figure 4. Example of hydrograph separation using R package 'EcoHydRology' for the watershed in our sample with the largest drainage area, Salado Creek in San Antonio, TX (drainage area = 505.8 km²). The flow response to a 38.1 mm (1.5 inch) total rainfall depth is shown. The response returns to baseflow conditions within the 36 hour period.
FIGURE 5: Example plots of linear and nonlinear relationships between rainfall and runoff (log transformed). Gray areas represent 90% confidence interval of model fit with both linear and nonlinear terms included, using robust standard estimates. The solid line is the predicted relationship with both linear and nonlinear terms included. The dashed line is the predicted relationship with only the linear term included.
FIGURE 6 Classification of all analysis basins included in this study based on model improvement with inclusion of seasonal controls, significance of nonlinear term (evidence of VSA behavior), and significance and sign of estimated coefficients associated with effect of rainfall-runoff ratio relationship during the growing season (April – September). Of catchments whose models were improved by controlling for seasonality and had significant individual coefficients, the highest frequency that appeared were for VSA catchments with positive coefficients for the nonlinear seasonal term. This is in agreement with previous theory and findings that VSA response should be more pronounced during the growing season.
Table 1. Estimated linear and nonlinear coefficients and robust standard errors for Baltimore Metropolitan Area watersheds

<table>
<thead>
<tr>
<th>Station ID</th>
<th>Name</th>
<th>Drainage area (km²)</th>
<th>Pct Dev</th>
<th>Linear Coefficient</th>
<th>Non-linear Coefficient</th>
</tr>
</thead>
<tbody>
<tr>
<td>1581500</td>
<td>BNUM RUN AT BEL AIR, MD</td>
<td>21.7</td>
<td>65.96</td>
<td>0.537</td>
<td>1.045</td>
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<td>1581752</td>
<td>PLUMTREE RUN NEAR BEL AIR, MD</td>
<td>6.5</td>
<td>78.88</td>
<td>0.690</td>
<td>0.874</td>
</tr>
<tr>
<td>1581757</td>
<td>OTTER POINT CREEK NEAR EDGEWOOD, MD</td>
<td>139</td>
<td>32.08</td>
<td>0.463</td>
<td>1.208</td>
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<tr>
<td>1583600</td>
<td>BEAVERDAM RUN AT COCKEYSVILLE, MD</td>
<td>53.6</td>
<td>51.17</td>
<td>0.271</td>
<td>0.706</td>
</tr>
<tr>
<td>1585090</td>
<td>WHITEMARSH RUN NEAR FULLERTON, MD</td>
<td>6.9</td>
<td>87.64</td>
<td>0.791</td>
<td>0.825</td>
</tr>
<tr>
<td>1585100</td>
<td>WHITEMARSH RUN AT WHITE MARSH, MD</td>
<td>19.7</td>
<td>84.78</td>
<td>0.727</td>
<td>0.940</td>
</tr>
<tr>
<td>1585104</td>
<td>HONEYGO RUN NEAR WHITE MARSH, MD</td>
<td>6.1</td>
<td>68.19</td>
<td>0.623</td>
<td>0.988</td>
</tr>
<tr>
<td>1585200</td>
<td>WEST BRANCH HERRING RUN AT IDLEWYLDE, MD</td>
<td>6</td>
<td>86.64</td>
<td>0.817</td>
<td>0.304</td>
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<tr>
<td>1589100</td>
<td>EAST BRANCH HERBERT RUN AT ARBUTUS, MD</td>
<td>6.4</td>
<td>91.3</td>
<td>0.930</td>
<td>0.030</td>
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<tr>
<td>1589197</td>
<td>GWYNNS FALLS NEAR DELIGHT, MD</td>
<td>10.6</td>
<td>78.43</td>
<td>0.730</td>
<td>0.424</td>
</tr>
<tr>
<td>1589290</td>
<td>SCOTTS LEVEL BRANCH AT ROCKDALE, MD</td>
<td>8.7</td>
<td>79.51</td>
<td>0.627</td>
<td>0.915</td>
</tr>
<tr>
<td>1589300</td>
<td>GWYNNS FALLS AT VILLA NOVA, MD</td>
<td>84.5</td>
<td>65.79</td>
<td>0.561</td>
<td>1.001</td>
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<tr>
<td>1589305</td>
<td>POWDER MILL RUN NEAR LOCHEARN, MD</td>
<td>9.2</td>
<td>89.08</td>
<td>0.848</td>
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<tr>
<td>1589312</td>
<td>DEAD RUN NEAR CATONSVILLE, MD</td>
<td>2</td>
<td>95.63</td>
<td>0.991</td>
<td>0.256</td>
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<tr>
<td>1589317</td>
<td>TRIBUTARY TO DEAD RUN TRIBUTARY AT WOODLAWN, MD</td>
<td>1.2</td>
<td>96.87</td>
<td>1.000</td>
<td>0.302</td>
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<tr>
<td>1589330</td>
<td>DEAD RUN AT FRANKLINTOWN, MD</td>
<td>14.2</td>
<td>95.2</td>
<td>0.969</td>
<td>0.502</td>
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<tr>
<td>1589352</td>
<td>GWYNNS FALLS AT WASHINGTON BLVD AT BALTIMORE, MD</td>
<td>159.1</td>
<td>75.72</td>
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<td>1589440</td>
<td>JONES FALLS AT SORRENTO, MD</td>
<td>65.1</td>
<td>33.9</td>
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<td>1.010</td>
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<tr>
<td>1589500</td>
<td>SAWMILL CREEK AT GLEN BURNIE, MD</td>
<td>12.6</td>
<td>66.55</td>
<td>0.446</td>
<td>0.563</td>
</tr>
<tr>
<td>1589795</td>
<td>SOUTH FORK JABEZ BRANCH AT MILLERSVILLE, MD</td>
<td>2.5</td>
<td>35.6</td>
<td>0.213</td>
<td>1.769</td>
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<tr>
<td>1593500</td>
<td>LITTLE PATUXENT RIVER AT GUILFORD, MD</td>
<td>98</td>
<td>58.66</td>
<td>0.445</td>
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<tr>
<td>1594000</td>
<td>LITTLE PATUXENT RIVER AT SAVAGE, MD</td>
<td>254.4</td>
<td>37.02</td>
<td>0.468</td>
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<tr>
<td>1594440</td>
<td>PATUXENT RIVER NEAR BOWIE, MD</td>
<td>906.6</td>
<td>31.86</td>
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</tr>
<tr>
<td>Site Code</td>
<td>Location Description</td>
<td>Temp</td>
<td>Precip</td>
<td>Temp Coeff</td>
<td>Precip Coeff</td>
</tr>
<tr>
<td>-----------</td>
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<td>------</td>
<td>--------</td>
<td>------------</td>
<td>--------------</td>
</tr>
<tr>
<td>1594526</td>
<td>WESTERN BRANCH AT UPPER MARLBORO, MD</td>
<td>233.6</td>
<td>50.38</td>
<td>0.610</td>
<td>0.05*</td>
</tr>
<tr>
<td>1644280</td>
<td>BROAD RUN NEAR LEESBURG, VA</td>
<td>196.9</td>
<td>56.03</td>
<td>0.299</td>
<td>0.11*</td>
</tr>
<tr>
<td>1644375</td>
<td>LITTLE SENECA CREEK TRIBUTARY NEAR GERMANTOWN, MD</td>
<td>3.3</td>
<td>82.65</td>
<td>0.625</td>
<td>0.09*</td>
</tr>
<tr>
<td>1645000</td>
<td>SENECA CREEK AT DAWSONVILLE, MD</td>
<td>262.4</td>
<td>36.92</td>
<td>0.296</td>
<td>0.07*</td>
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<tr>
<td>1646000</td>
<td>DIFFICULT RUN NEAR GREAT FALLS, VA</td>
<td>149.9</td>
<td>50.53</td>
<td>0.623</td>
<td>0.09*</td>
</tr>
<tr>
<td>1647850</td>
<td>TURKEY BRANCH NEAR ROCKVILLE, MD</td>
<td>7</td>
<td>88.48</td>
<td>0.968</td>
<td>0.13*</td>
</tr>
<tr>
<td>1648000</td>
<td>ROCK CREEK AT SHERILL DRIVE WASHINGTON, DC</td>
<td>136.8</td>
<td>70.99</td>
<td>0.715</td>
<td>0.11*</td>
</tr>
<tr>
<td>1649150</td>
<td>PAINT BRANCH TRIBUTARY NEAR COLESVILLE, MD</td>
<td>2.7</td>
<td>39.35</td>
<td>0.456</td>
<td>0.08*</td>
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<tr>
<td>1649190</td>
<td>PAINT BRANCH NEAR COLLEGE PARK, MARYLAND</td>
<td>34</td>
<td>57.34</td>
<td>0.483</td>
<td>0.09*</td>
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<tr>
<td>1649500</td>
<td>NORTH EAST BRANCH ANACOSTIA RIVER AT RIVERDALE, MD</td>
<td>188.1</td>
<td>62.92</td>
<td>0.567</td>
<td>0.08*</td>
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<tr>
<td>1650500</td>
<td>NW BRANCH ANACOSTIA RIVER NEAR COLESVILLE, MD</td>
<td>54.8</td>
<td>48.29</td>
<td>0.665</td>
<td>0.11*</td>
</tr>
</tbody>
</table>

* indicates coefficient estimate is significant at the alpha = 0.05 level.
### TABLE 2. Results of logistic regression of percent undeveloped land and other controls on probability of VSA-type response

**Panel A: National Dataset**

<table>
<thead>
<tr>
<th>Variable</th>
<th>Estimate</th>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
<th>Panel B: BMA Dataset</th>
</tr>
</thead>
<tbody>
<tr>
<td>Undeveloped Land (%)</td>
<td>0.034</td>
<td>3.476</td>
<td>1.32</td>
<td>0.012</td>
</tr>
<tr>
<td>Total Impervious Area (%)</td>
<td></td>
<td></td>
<td></td>
<td>0.012</td>
</tr>
<tr>
<td>Developed Open Space (%)</td>
<td>-0.032</td>
<td>-3.154</td>
<td>-0.502</td>
<td>0.000</td>
</tr>
<tr>
<td>Distance to CSO (m)</td>
<td></td>
<td>0.000</td>
<td>0.535</td>
<td>0.000</td>
</tr>
<tr>
<td>Ret/Det SW Infrastructure (binary)</td>
<td>0.301</td>
<td>35.081</td>
<td>0.459</td>
<td>1.483</td>
</tr>
<tr>
<td>Average Slope (%)</td>
<td>-0.441</td>
<td>-2.225</td>
<td>0.133</td>
<td>0.301</td>
</tr>
<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>0.038</td>
<td>3.892</td>
<td>0.535</td>
<td>1.483</td>
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<tr>
<td>Catchment Area (km²)</td>
<td>0.002</td>
<td>0.152</td>
<td>0.448</td>
<td>0.560</td>
</tr>
<tr>
<td>Intercept</td>
<td>0.516</td>
<td>67.566</td>
<td>0.185</td>
<td>0.670</td>
</tr>
</tbody>
</table>

**Panel B: BMA Dataset**

<table>
<thead>
<tr>
<th>Variable</th>
<th>Estimate</th>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
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<td>Undeveloped Land (%)</td>
<td>0.053</td>
<td>5.488</td>
<td>2.392 *</td>
<td>4</td>
<td>0.066</td>
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<tr>
<td>Total Impervious Area (%)</td>
<td>0.012</td>
<td>1.249</td>
<td>0.133</td>
<td>9</td>
<td>0.012</td>
</tr>
<tr>
<td>Developed Open Space (%)</td>
<td>-0.032</td>
<td>-3.154</td>
<td>-0.502</td>
<td>6.655</td>
<td>0.000</td>
</tr>
<tr>
<td>Distance to CSO (m)</td>
<td>0.000</td>
<td>0.000</td>
<td>0.535</td>
<td>1.965</td>
<td>0.000</td>
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<tr>
<td>Ret/Det SW Infrastructure (binary)</td>
<td>0.301</td>
<td>35.081</td>
<td>0.459</td>
<td>1.483</td>
<td>0.301</td>
</tr>
<tr>
<td>Average Slope (%)</td>
<td>-0.298</td>
<td>25.763</td>
<td>-1.332</td>
<td>1.670</td>
<td>0.301</td>
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<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>0.036</td>
<td>3.692</td>
<td>1.456</td>
<td>1.694</td>
<td>0.036</td>
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<tr>
<td>Catchment Area (km²)</td>
<td>0.000</td>
<td>0.015</td>
<td>0.036</td>
<td>1.547</td>
<td>0.000</td>
</tr>
<tr>
<td>Intercept</td>
<td>-3.160</td>
<td>-96</td>
<td>-1.491</td>
<td>1.547</td>
<td>-3.270</td>
</tr>
</tbody>
</table>

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[McFadden’s R²](http://mc.manuscriptcentral.com/hyp) | 0.038 | 0.124 | 0.143 |

[Count-based R² (above mean)](http://mc.manuscriptcentral.com/hyp) | 0.560 | 0.670 | 0.692 |
### Panel C: non-CSS Dataset (n = 56)(c)

<table>
<thead>
<tr>
<th></th>
<th>Estimate (%)</th>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
<th>VIF</th>
<th>Estimate (%)</th>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
<th>VIF</th>
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<tbody>
<tr>
<td>Undeveloped Land (%)</td>
<td>0.036</td>
<td>3.668</td>
<td>1.444</td>
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<td>0.087</td>
<td>9.039</td>
<td>2.186</td>
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<td>Total Impervious Area (%)</td>
<td>-0.046</td>
<td>-4.475</td>
<td>-2.756</td>
<td></td>
<td>-0.046</td>
<td>71.09</td>
<td>2.756</td>
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<tr>
<td>Developed Open Space (%)</td>
<td>-1.207</td>
<td>-12.367</td>
<td>-3.394</td>
<td></td>
<td>-1.207</td>
<td>-12.367</td>
<td>-3.394</td>
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</tr>
<tr>
<td>Distance to CSO (m)</td>
<td>-0.452</td>
<td>-36.388</td>
<td>-1.465</td>
<td></td>
<td>-0.452</td>
<td>-36.388</td>
<td>-1.465</td>
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<tr>
<td>Average Slope (%)</td>
<td>0.075</td>
<td>80.000</td>
<td>70.09</td>
<td></td>
<td>-0.046</td>
<td>-2.475</td>
<td>-1.793</td>
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<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>-0.132</td>
<td>-12.367</td>
<td>-0.509</td>
<td>25.00</td>
<td>-0.132</td>
<td>36.388</td>
<td>-1.465</td>
<td>15.03</td>
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<td>Catchment Area (km²)</td>
<td>0.004</td>
<td>-0.004</td>
<td>-0.048</td>
<td></td>
<td>0.004</td>
<td>0.004</td>
<td>2.157</td>
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<td>Intercept</td>
<td>-0.415</td>
<td>33.942</td>
<td>-0.601</td>
<td></td>
<td>-85.310</td>
<td>-100</td>
<td>-2.648</td>
<td></td>
</tr>
<tr>
<td>McFadden’s R²</td>
<td>0.051</td>
<td>0.417</td>
<td>0.672</td>
<td></td>
<td>0.618</td>
<td>0.853</td>
<td>0.941</td>
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<tr>
<td>Count-based R² (above mean)</td>
<td>0.618</td>
<td>0.853</td>
<td>0.941</td>
<td></td>
<td>0.618</td>
<td>0.853</td>
<td>0.941</td>
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<tr>
<td>Catchment Area (km²)</td>
<td>103.19</td>
<td>-0.002</td>
<td>0.191</td>
<td>-0.373</td>
<td>1.46</td>
<td>0</td>
<td>-0.004</td>
<td>-0.371</td>
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<td>----------------------</td>
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<td>--------</td>
<td>--------</td>
<td>--------</td>
<td>------</td>
<td>---</td>
<td>--------</td>
<td>--------</td>
</tr>
<tr>
<td>Intercept</td>
<td>0.709</td>
<td>6</td>
<td>1.459</td>
<td>7.285</td>
<td>32</td>
<td>1.539</td>
<td>3.932</td>
<td>5003</td>
</tr>
<tr>
<td>McFadden’s R²</td>
<td>0.040</td>
<td>0.241</td>
<td>0.325</td>
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<td></td>
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<tr>
<td>Count-based R² (above mean)</td>
<td>0.536</td>
<td>0.768</td>
<td>0.821</td>
<td></td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

(a) Significance at the 0.10 level; * Significance at the 0.05 level; ** Significance at the 0.01 level
(b) Retention/Detention stormwater infrastructure excluded because all BMA watersheds located in counties with detention, retention or infiltration-based infrastructure
(c) Distance to CSO and Ret/Det stormwater infrastructure effects could not be estimated due to complete separation in the data
Table 3. Results of logistic regression of development types and other controls on probability of VSA-type response

<table>
<thead>
<tr>
<th>Panel A: National Dataset (n = 91)</th>
<th>MODEL 2A: TIA</th>
<th>MODEL2B: TIA + Open Space</th>
<th>MODEL2C: TIA + Open Space + Other Development Characteristics</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Estimate</td>
<td>Effect on Odds (%)</td>
<td>t-statistic</td>
</tr>
<tr>
<td>Total Impervious Area (%)</td>
<td>-0.023</td>
<td>-2.235</td>
<td>-0.861</td>
</tr>
<tr>
<td>Developed Open Space (%)</td>
<td>-0.089</td>
<td>-8.554</td>
<td>-2.364</td>
</tr>
<tr>
<td>Distance to CSO (m)</td>
<td>0.000</td>
<td>0.000</td>
<td>0.789</td>
</tr>
<tr>
<td>Ret/Det SW Infrastructure (binary)</td>
<td>0.120</td>
<td>12.776</td>
<td>0.190</td>
</tr>
<tr>
<td>Average Slope (%)</td>
<td>-0.327</td>
<td>-27.879</td>
<td>-1.613</td>
</tr>
<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>0.036</td>
<td>3.646</td>
<td>1.919</td>
</tr>
<tr>
<td>Catchment Area (km²)</td>
<td>0.003</td>
<td>0.350</td>
<td>0.290</td>
</tr>
<tr>
<td>Intercept</td>
<td>-1.621</td>
<td>-80.234</td>
<td>-0.781</td>
</tr>
<tr>
<td>McFadden’s R²</td>
<td>0.066</td>
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</tr>
<tr>
<td>Count-based R² (above mean)</td>
<td>0.626</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Panel B: BMA Dataset (n = 34)

|                                   | Estimate       | Effect on Odds (%)        | t-statistic | VIF | Estimate       | Effect on Odds (%) | t-statistic | VIF | Estimate       | Effect on Odds (%) | t-statistic | VIF |
| Total Impervious Area (%)         | -0.117         | -11.081                   | -2.052      | * 14.0| -0.197         | -17.855           | -2.154      | * 4.617| -0.438         | -35.489           | 2.160      | * 0 |

http://mc.manuscriptcentral.com/hyp
<table>
<thead>
<tr>
<th>Developed Open Space (%)</th>
<th>-0.163</th>
<th>-15.022</th>
<th>-1.502</th>
<th>2.695</th>
<th>-0.441</th>
<th>-35.647</th>
<th>1.818</th>
<th>8</th>
<th>-4.34</th>
</tr>
</thead>
<tbody>
<tr>
<td>Distance to CSO (m)</td>
<td>0.000</td>
<td>-0.029</td>
<td>1.932</td>
<td></td>
<td>-2.060</td>
<td>-87.254</td>
<td>1.938</td>
<td>0</td>
<td>2.56</td>
</tr>
<tr>
<td>Average Slope (%)</td>
<td>-1.456</td>
<td>-76.679</td>
<td>-1.938</td>
<td>-1.711</td>
<td>-1.965</td>
<td>2.913</td>
<td>-0.606</td>
<td>8</td>
<td>5.24</td>
</tr>
<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>0.616</td>
<td>85.128</td>
<td>2.608</td>
<td>0.769</td>
<td>116</td>
<td>2.743</td>
<td>3.192</td>
<td>2</td>
<td>2.34</td>
</tr>
<tr>
<td>Catchment Area (km²)</td>
<td>0.007</td>
<td>0.663</td>
<td>1.021</td>
<td>0.002</td>
<td>0.152</td>
<td>0.208</td>
<td>1.882</td>
<td>0.003</td>
<td>0.281</td>
</tr>
</tbody>
</table>
| Intercept                | -63.220| 100.00  | -2.551 | *    | -73.528| -100   | -2.682| * | -83.951| -100 | 2.448 | *

**McFadden’s R²**

| Count-based R² (above mean) | 0.794 | 0.824 | 0.853 |

### Panel C: non-CS Dataset (n = 56)

<table>
<thead>
<tr>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
<th>VIF</th>
<th>Effect on Odds (%)</th>
<th>t-statistic</th>
<th>VIF</th>
</tr>
</thead>
<tbody>
<tr>
<td>Total Impervious Area (%)</td>
<td>0.013</td>
<td>-1.284</td>
<td>-0.321</td>
<td>0</td>
<td>-0.132</td>
</tr>
<tr>
<td>Developed Open Space (%)</td>
<td>0.015</td>
<td>-14.477</td>
<td>-2.037</td>
<td>0</td>
<td>-1.037</td>
</tr>
<tr>
<td>Average Slope (%)</td>
<td>-0.762</td>
<td>-53.331</td>
<td>-2.037</td>
<td>*</td>
<td>0</td>
</tr>
<tr>
<td>Average Annual Precipitation (cm/yr)</td>
<td>0.007</td>
<td>0.406</td>
<td>0.831</td>
<td>3</td>
<td>-0.001</td>
</tr>
</tbody>
</table>
| Catchment Area (km²) | 5.014 | 14952 | 1.114 | 13.245 | 40 | 2.001 | *
| Intercept | 0.131 | 0.9 | 0.82 |
| McFadden’s R² | 0.696 | 0.29 | 1 |
(a) Significance at the 0.10 level; * Significance at the 0.05 level; ** Significance at the 0.01 level
(b) Retention/Detention stormwater infrastructure excluded because all BMA watersheds located in counties with detention, retention or infiltration-based infrastructure
(c) Distance to CSO and Ret/Det stormwater infrastructure effects could not be estimated due to complete separation in the data